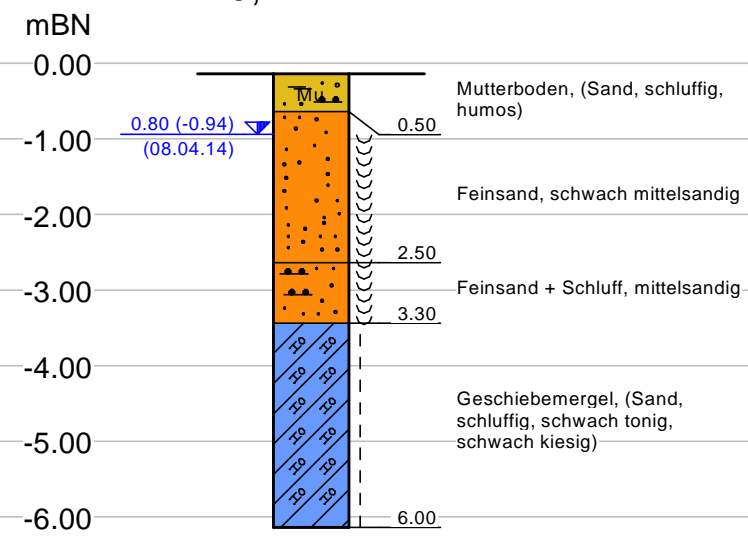


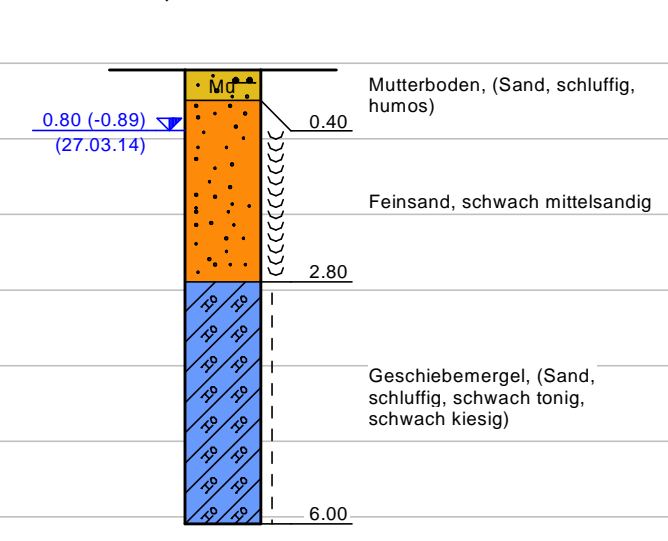
### BS 1

-0,14 mBN



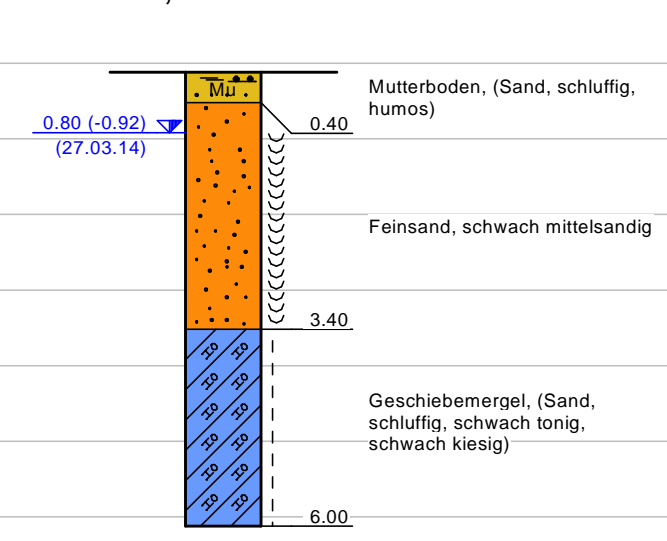
### BS 2

-0,09 mBN



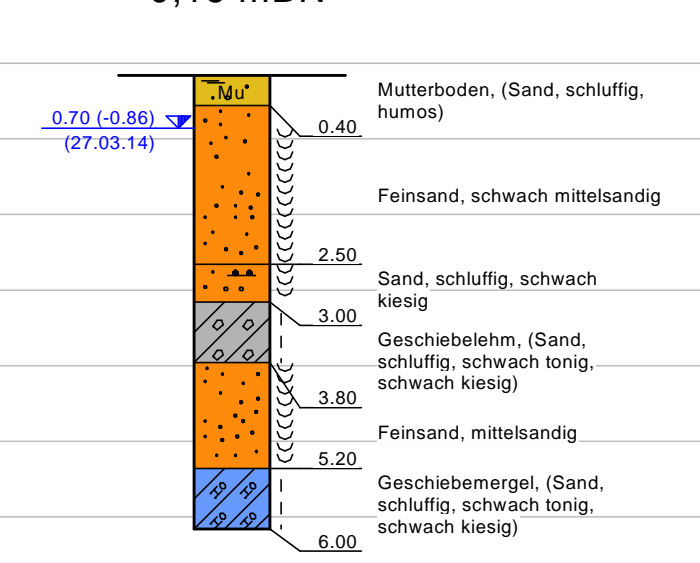
### BS 3

-0,12 mBN



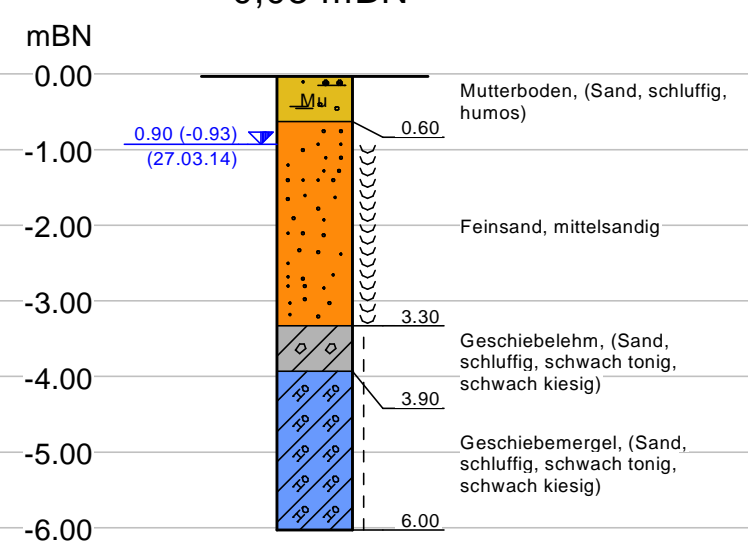
### BS 4

-0,16 mBN



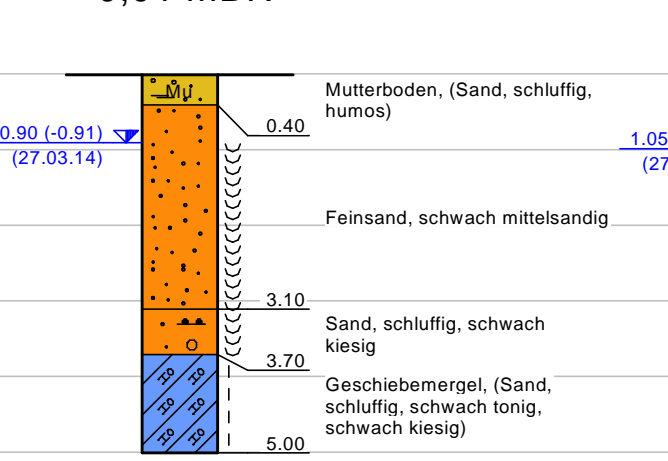
### BS 5

-0,03 mBN



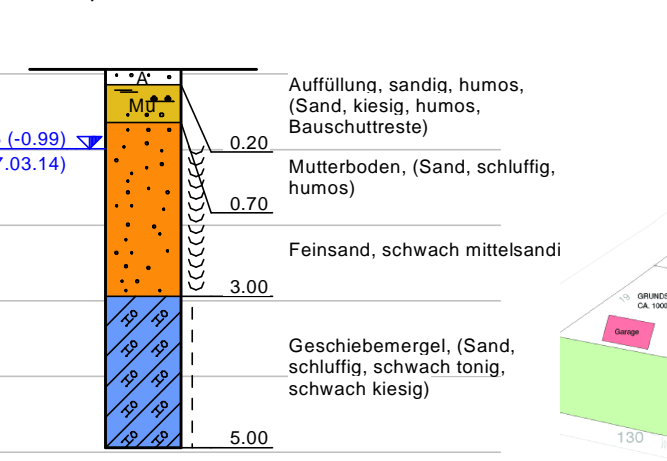
### BS 6

-0,01 mBN



### BS 7

+0,06 mBN



**Legende**

steif  
naß

2.45  
30.04.98 GW angebohrt  
2.45  
30.04.98 GW Bohrende

**Beratende Ingenieure und Geologen Rohde und Schlesch**  
Pinneberger Str. 5b, 25436 Tornesch  
Tel. 04122 / 960 399 Fax: 960 402

Projekt-Nr.: 1041/14  
Anlage: 1  
Mdh = 1:100  
17.04.14  
Datei: BS 1-7.bop

BV Heistmer Weg  
25436 Moorrege

**Bodenprofile BS1 - BS7**

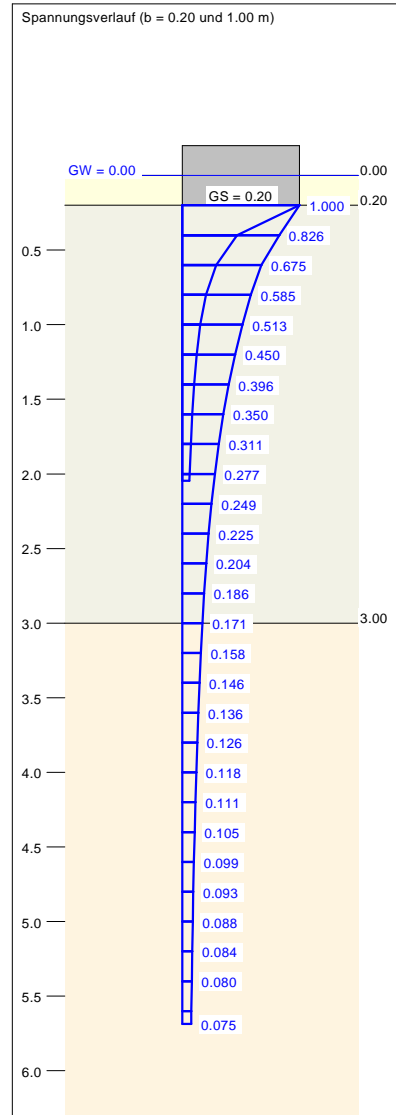
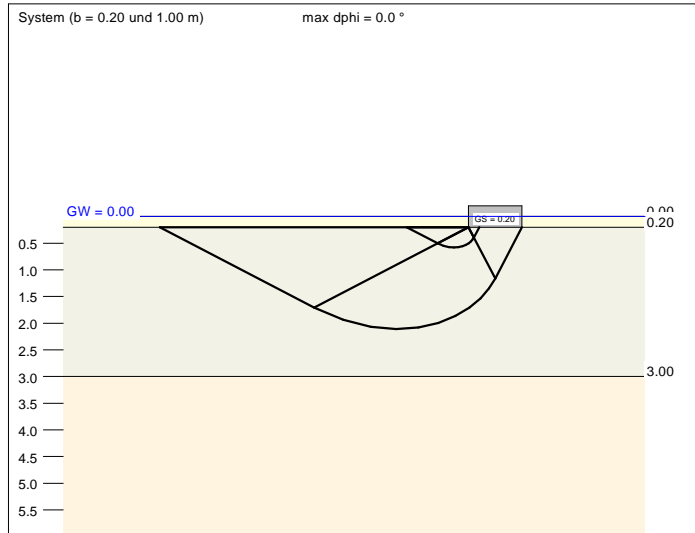


**Sohldruck und Setzung**  
**Streifenfundamente d = 20 cm**  
 BV Heistmer Weg Moorrege

Bericht Nr. 1041/14

Anlage Nr. 2.1

| Boden | $\gamma$<br>[kN/m <sup>3</sup> ] | $\gamma'$<br>[kN/m <sup>3</sup> ] | $\phi$<br>[°] | c<br>[kN/m <sup>2</sup> ] | $E_s$<br>[MN/m <sup>2</sup> ] | $\nu$<br>[-] | Bezeichnung |
|-------|----------------------------------|-----------------------------------|---------------|---------------------------|-------------------------------|--------------|-------------|
|       | 25.0                             | 15.0                              | 0.0           | 0.0                       | 100.0                         | 0.00         | Beton       |
|       | 19.0                             | 11.0                              | 35.0          | 0.0                       | 40.0                          | 0.00         | Sand        |
|       | 22.0                             | 12.0                              | 30.0          | 10.0                      | 40.0                          | 0.00         | Gmg         |

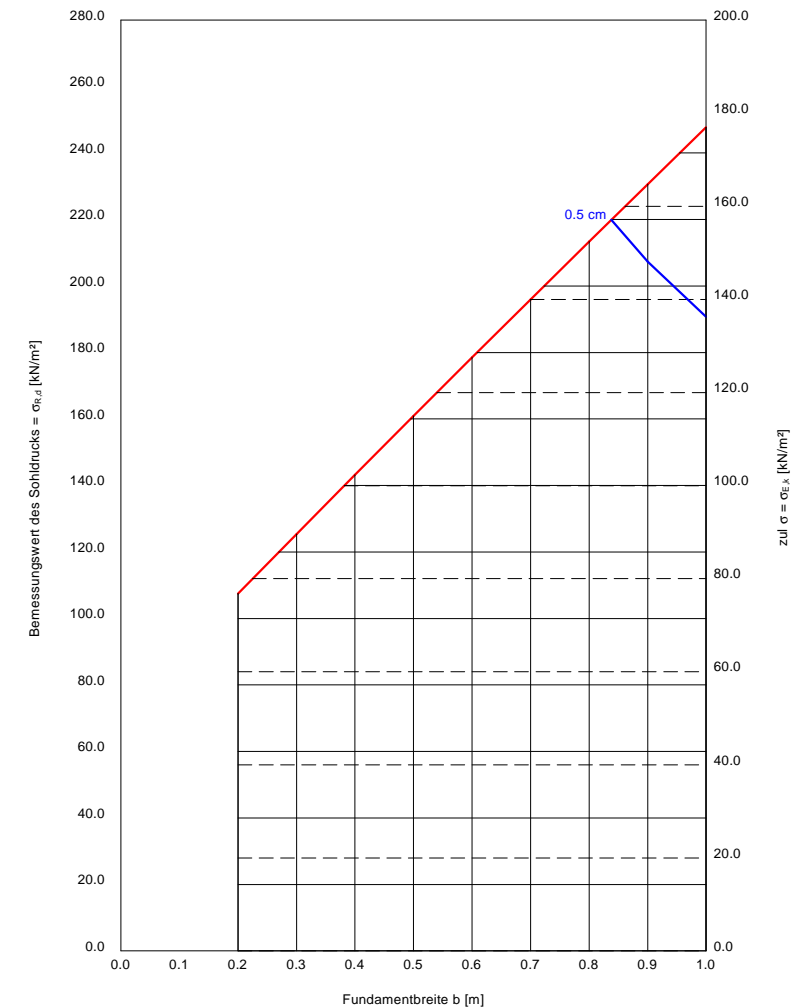


Berechnungsgrundlagen:  
 Beispiel "Mehrere Fundamente"  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 10.00 m)  
 $\gamma_{Gr} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 $\gamma_{(G,Q)} = 0.330 \cdot \gamma_Q + (1 - 0.330) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.399$

Anteil Veränderliche Lasten = 0.330  
 Gründungssohle = 0.20 m  
 Grundwasser = 0.00 m  
 Grenztiefe mit p = 20.0 %  
 — Sohlldruck  
 — Setzungen

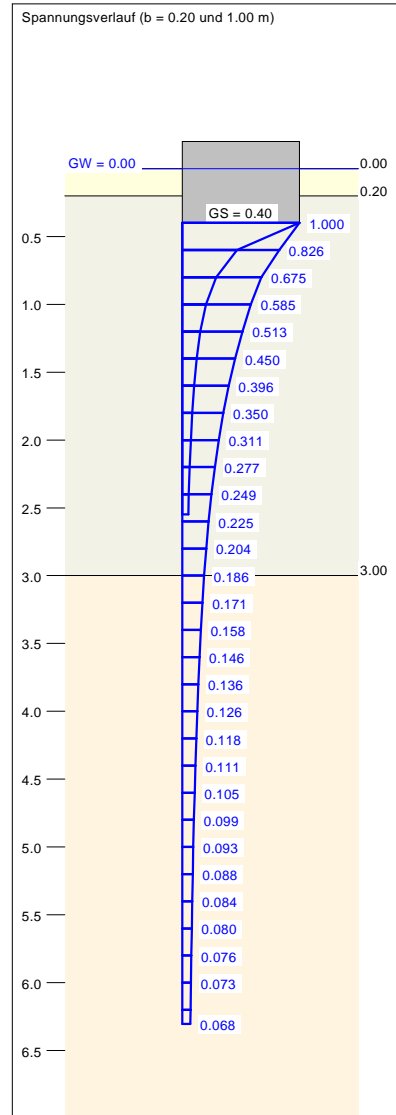
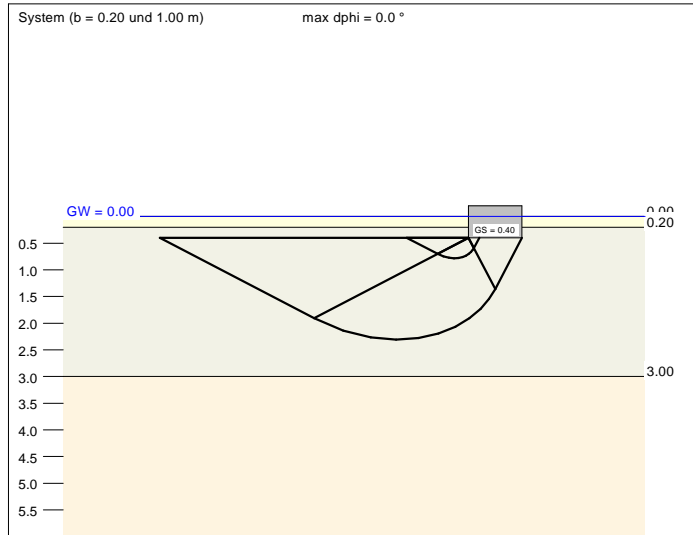
| a<br>[m] | b<br>[m] | $\sigma_{G,k}$<br>[kN/m <sup>2</sup> ] | $\sigma_{R,d}$<br>[kN/m <sup>2</sup> ] | $R_{a,d}$<br>[kN/m] | Zul $\sigma' / \sigma_{E,k}$<br>[kN/m <sup>2</sup> ] | s<br>[cm] | cal $\phi$<br>[°] | cal c<br>[kN/m <sup>2</sup> ] | $\gamma_2$<br>[kN/m <sup>3</sup> ] | $\sigma_{\phi}$<br>[kN/m <sup>2</sup> ] | $t_{\phi}$<br>[m] | UK LS<br>[m] |
|----------|----------|--|--|---------------------|--|-----------|-------------------|-------------------------------|------------------------------------|---|-------------------|--------------|
| 10.00    | 0.20     | 150.5                                  | 107.5                                  | 21.5                | 76.8   | 0.07      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 2.05              | 0.58         |
| 10.00    | 0.30     | 175.6                                  | 125.4                                  | 37.6                | 89.6   | 0.12      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 2.60              | 0.77         |
| 10.00    | 0.40     | 200.5                                  | 143.2                                  | 57.3                | 102.3  | 0.18      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 3.10              | 0.96         |
| 10.00    | 0.50     | 225.3                                  | 160.9                                  | 80.5                | 115.0  | 0.24      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 3.57              | 1.15         |
| 10.00    | 0.60     | 249.9                                  | 178.5                                  | 107.1               | 127.5  | 0.31      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 4.01              | 1.34         |
| 10.00    | 0.70     | 274.4                                  | 196.0                                  | 137.2               | 140.0  | 0.39      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 4.45              | 1.54         |
| 10.00    | 0.80     | 298.7                                  | 213.4                                  | 170.7               | 152.5  | 0.47      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 4.87              | 1.73         |
| 10.00    | 0.90     | 322.9                                  | 230.6                                  | 207.6               | 164.8  | 0.56      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 5.28              | 1.92         |
| 10.00    | 1.00     | 346.9                                  | 247.8                                  | 247.8               | 177.1  | 0.65      | 35.0              | 0.00                          | 11.00                              | 3.00                                    | 5.69              | 2.11         |

Zul  $\sigma = \sigma_{E,k} = \sigma_{G,k} / (\gamma_{Gr} \cdot \gamma_{(G,Q)}) = \sigma_{G,k} / (1.40 \cdot 1.40) = \sigma_{G,k} / 1.96$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.33





| Boden | $\gamma$<br>[kN/m <sup>3</sup> ] | $\gamma'$<br>[kN/m <sup>3</sup> ] | $\phi$<br>[°] | c<br>[kN/m <sup>2</sup> ] | $E_s$<br>[MN/m <sup>2</sup> ] | $\nu$<br>[-] | Bezeichnung |
|-------|----------------------------------|-----------------------------------|---------------|---------------------------|-------------------------------|--------------|-------------|
|       | 25.0                             | 15.0                              | 0.0           | 0.0                       | 100.0                         | 0.00         | Beton       |
|       | 19.0                             | 11.0                              | 35.0          | 0.0                       | 40.0                          | 0.00         | Sand        |
|       | 22.0                             | 12.0                              | 30.0          | 10.0                      | 40.0                          | 0.00         | Gmg         |

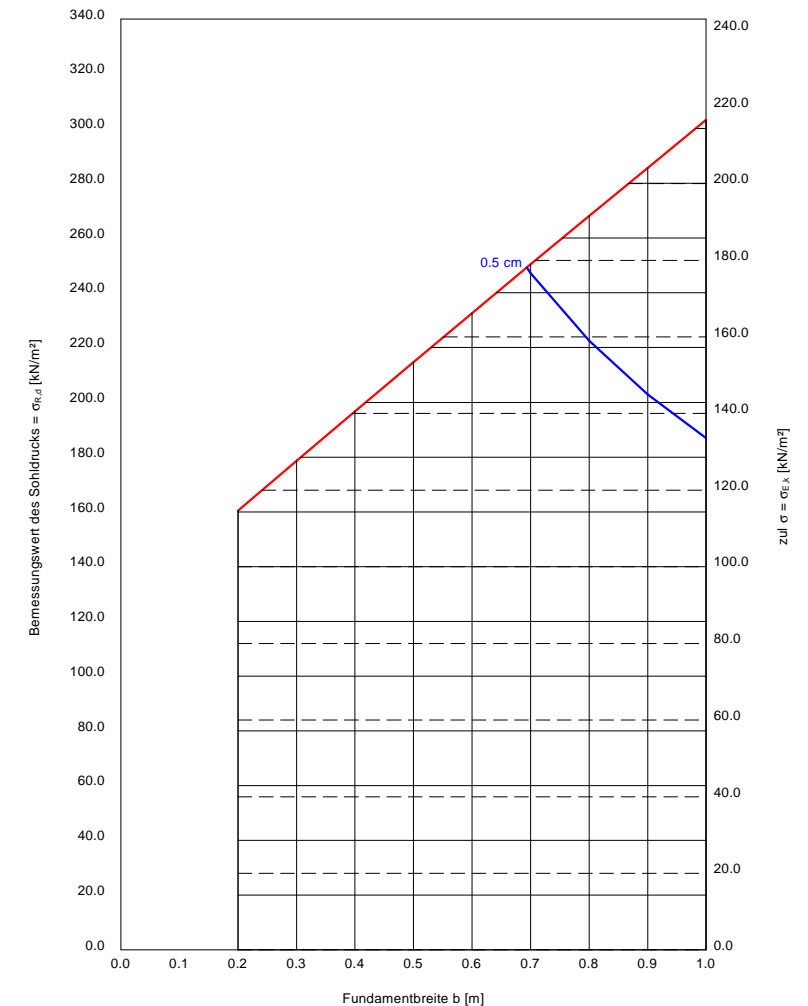


Berechnungsgrundlagen:  
 Beispiel "Mehrere Fundamente"  
 Grundbruchformel nach DIN 4017:2006  
 Teilsicherheitskonzept (EC 7)  
 Streifenfundament (a = 10.00 m)

Anteil Veränderliche Lasten = 0.330  
 Gründungssohle = 0.40 m  
 Grundwasser = 0.00 m  
 Grenztiefe mit p = 20.0 %

$\gamma_{Gr} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 $\gamma_{(G,Q)} = 0.330 \cdot \gamma_Q + (1 - 0.330) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.399$

— Sohlldruck  
 — Setzungen



| a<br>[m] | b<br>[m] | $\sigma_{G,k}$<br>[kN/m <sup>2</sup> ] | $\sigma_{R,d}$<br>[kN/m <sup>2</sup> ] | $R_{0,d}$<br>[kN/m] | Zul $\sigma/\sigma_{G,k}$<br>[kN/m <sup>2</sup> ] | s<br>[cm] | cal $\phi$<br>[°] | cal c<br>[kN/m <sup>2</sup> ] | $\gamma_2$<br>[kN/m <sup>3</sup> ] | $\sigma_0$<br>[kN/m <sup>2</sup> ] | $t_g$<br>[m] | UK LS<br>[m] |
|----------|----------|--|--|---------------------|---|-----------|-------------------|-------------------------------|------------------------------------|------------------------------------|--------------|--------------|
| 10.00    | 0.20     | 224.6                                  | 160.4                                  | 32.1                | 114.6   | 0.12      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 2.55         | 0.78         |
| 10.00    | 0.30     | 250.1                                  | 178.6                                  | 53.6                | 127.6   | 0.18      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 3.14         | 0.97         |
| 10.00    | 0.40     | 275.4                                  | 196.7                                  | 78.7                | 140.6   | 0.25      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 3.65         | 1.16         |
| 10.00    | 0.50     | 300.6                                  | 214.7                                  | 107.4               | 153.4   | 0.33      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 4.14         | 1.35         |
| 10.00    | 0.60     | 325.7                                  | 232.6                                  | 139.6               | 166.2   | 0.42      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 4.60         | 1.54         |
| 10.00    | 0.70     | 350.6                                  | 250.4                                  | 175.3               | 178.9   | 0.51      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 5.05         | 1.74         |
| 10.00    | 0.80     | 375.3                                  | 268.1                                  | 214.5               | 191.6   | 0.60      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 5.48         | 1.93         |
| 10.00    | 0.90     | 399.9                                  | 285.7                                  | 257.1               | 204.1   | 0.70      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 5.90         | 2.12         |
| 10.00    | 1.00     | 424.4                                  | 303.1                                  | 303.1               | 216.6   | 0.81      | 35.0              | 0.00                          | 11.00                              | 5.20                               | 6.30         | 2.31         |

Zul  $\sigma = \sigma_{G,k} = \sigma_{G,k} / (\gamma_{Gr} \cdot \gamma_{(G,Q)}) = \sigma_{G,k} / (1.40 \cdot 1.40) = \sigma_{G,k} / 1.96$  (für Setzungen)  
 Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.33

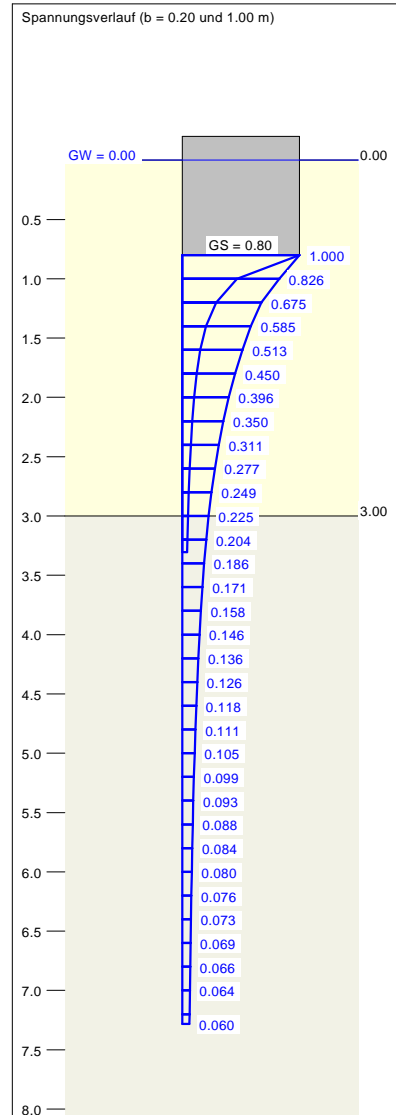
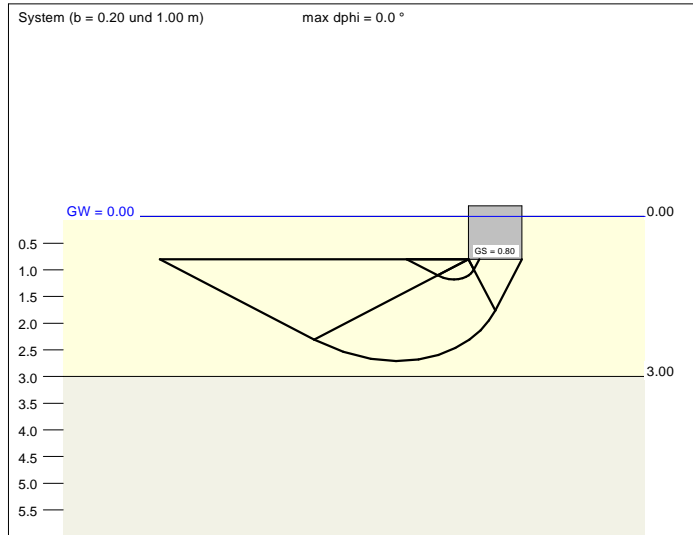


Sohldruck und Setzung  
Streifenfundamente d = 80 cm  
BV Heistmer Weg Moorrege

Bericht Nr. 1041/14

Anlage Nr. 2.3

| Boden | $\gamma$<br>[kN/m <sup>3</sup> ] | $\gamma'$<br>[kN/m <sup>3</sup> ] | $\phi$<br>[°] | c<br>[kN/m <sup>2</sup> ] | $E_s$<br>[MN/m <sup>2</sup> ] | $\nu$<br>[-] | Bezeichnung |
|-------|----------------------------------|-----------------------------------|---------------|---------------------------|-------------------------------|--------------|-------------|
|       | 19.0                             | 11.0                              | 35.0          | 0.0                       | 40.0                          | 0.00         | Sand        |
|       | 22.0                             | 12.0                              | 30.0          | 10.0                      | 40.0                          | 0.00         | Gmg         |



Berechnungsgrundlagen:  
Beispiel "Mehrere Fundamente"  
Grundbruchformel nach DIN 4017:2006  
Teilsicherheitskonzept (EC 7)  
Streifenfundament (a = 10.00 m)  
 $\gamma_{Gr} = 1.40$   
 $\gamma_G = 1.35$   
 $\gamma_Q = 1.50$   
 $\gamma_{(G,Q)} = 0.330 \cdot \gamma_Q + (1 - 0.330) \cdot \gamma_G$   
 $\gamma_{(G,Q)} = 1.399$

Anteil Veränderliche Lasten = 0.330  
Gründungssohle = 0.80 m  
Grundwasser = 0.00 m  
Grenztiefe mit p = 20.0 %  
— Sohldruck  
— Setzungen

| a<br>[m] | b<br>[m] | $\sigma_{G,k}$<br>[kN/m <sup>2</sup> ] | $\sigma_{Q,k}$<br>[kN/m <sup>2</sup> ] | $R_{0,d}$<br>[kN/m] | Zul $\sigma/\sigma_{G,k}$<br>[kN/m <sup>2</sup> ] | s<br>[cm] | cal $\phi$<br>[°] | cal c<br>[kN/m <sup>2</sup> ] | $\gamma_2$<br>[kN/m <sup>2</sup> ] | $\sigma_0$<br>[kN/m <sup>2</sup> ] | $t_g$<br>[m] | UK LS<br>[m] |
|----------|----------|--|--|---------------------|---|-----------|-------------------|-------------------------------|------------------------------------|------------------------------------|--------------|--------------|
| 10.00    | 0.20     | 345.8                                  | 247.0                                  | 49.4                | 176.5   | 0.19      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 3.31         | 1.18         |
| 10.00    | 0.30     | 372.0                                  | 265.7                                  | 79.7                | 189.9   | 0.28      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 3.94         | 1.37         |
| 10.00    | 0.40     | 398.0                                  | 284.3                                  | 113.7               | 203.2   | 0.38      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 4.50         | 1.56         |
| 10.00    | 0.50     | 423.9                                  | 302.8                                  | 151.4               | 216.4   | 0.48      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 5.02         | 1.75         |
| 10.00    | 0.60     | 449.7                                  | 321.2                                  | 192.7               | 229.5   | 0.59      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 5.51         | 1.94         |
| 10.00    | 0.70     | 475.2                                  | 339.5                                  | 237.6               | 242.6   | 0.71      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 5.98         | 2.14         |
| 10.00    | 0.80     | 500.7                                  | 357.6                                  | 286.1               | 255.5   | 0.83      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 6.43         | 2.33         |
| 10.00    | 0.90     | 526.0                                  | 375.7                                  | 338.1               | 268.4   | 0.95      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 6.86         | 2.52         |
| 10.00    | 1.00     | 551.1                                  | 393.6                                  | 393.6               | 281.3   | 1.08      | 35.0              | 0.00                          | 11.00                              | 8.80                               | 7.28         | 2.71         |

Zul  $\sigma = \sigma_{G,k} = \sigma_{G,k} / (\gamma_{Gr} \cdot \gamma_{(G,Q)}) = \sigma_{G,k} / (1.40 \cdot 1.40) = \sigma_{G,k} / 1.96$  (für Setzungen)  
Verhältnis Veränderliche(Q)/Gesamtlaster(G+Q) [-] = 0.33

